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## Predicting daily streamflow in ungauged rural catchments: the case of Masinga catchment, Kenya

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**Abstract** Access to daily streamflow data at the catchment scale, is a central component of many aspects of water resources and water quality management. However, the majority of river reaches in many catchments in Kenya are ungauged or poorly gauged, and in some cases existing measurement networks are declining. Long-term continuous monitoring is not being done due to high costs of equipment maintenance. Therefore, there is a need for an alternative tool such as a catchment-scale hydrological model that is capable of predicting the daily streamflow. An approach is presented of predicting daily streamflow using a physically-based catchment-scale model, the geospatial Stream Flow Model (SFM). The SFM was developed using the “C” programming language and the user interface was developed using the Avenue script of the ArcView software. The SFM simulates the dynamics of runoff processes by utilizing remotely sensed and widely available global or local data sets. The model was applied in the Masinga catchment, Kenya, and the results gave a model performance coefficient of 0.74 based on the Nash-Sutcliffe statistical criterion.

**Key words** ArcView GIS; daily streamflow; hydrological modelling; Masinga catchment, Kenya; Stream Flow Model; ungauged catchment

### Prévision de l'écoulement journalier dans des bassins versants ruraux non jaugés: le cas du bassin versant de Masinga, Kenya

**Résumé** L'accès aux données d'écoulement journalier à l'échelle du bassin versant est une composante centrale de nombreux aspects de la gestion des ressources en eau et de la qualité de l'eau. Cependant, la majorité des cours d'eau de nombreux bassins du Kenya ne sont pas ou sont peu jaugés, et dans certains cas les réseaux de mesure existants sont en déclin. Le suivi continu sur le long terme n'est pas assuré à cause des coûts élevés de maintenance des équipements. Un outil alternatif est par conséquent nécessaire, comme un modèle hydrologique de bassin versant capable de prévoir l'écoulement journalier. Une approche de prévision de l'écoulement journalier est présentée, basée sur un modèle à bases physiques à l'échelle du bassin versant, le “geospatial Stream Flow Model” (SFM). Le SFM a été développé avec le langage de programmation C tandis que l'interface utilisateur a été développée avec le script Avenue du logiciel ArcView. Le SFM simule la dynamique des processus d'écoulement en utilisant des jeux de données globaux et locaux largement disponibles obtenus par télédétection. Le modèle a été appliqué au bassin versant de Masinga, au Kenya, et les résultats ont donné un coefficient de performance de modélisation de 0.74 basé sur le critère statistique de Nash-Sutcliffe.

**Mots clefs** SIG ArcView; écoulement journalier; modélisation hydrologique; bassin versant de Masinga, Kenya; Stream Flow Model; bassin non jaugé

## INTRODUCTION

Producing streamflow estimates for ungauged catchments has attracted a lot of interest among hydrologists and hydraulic engineers, but the problem still remains unresolved (Nandakumar & Mein, 1997). Estimates of the magnitude and frequency of streamflow are needed to safely and economically design hydraulic structures such as dams, bridges and culverts (Scott *et al.*, 2003). These estimates also are used for managing flood plains, identifying flood-hazard areas, and establishing flood-insurance rates, but may be required at ungauged sites where no observed flood data are available for streamflow-frequency analysis (Feaster & Tasker, 2002). The problem is compounded by the impacts of human-induced changes to the land surface and climate, occurring at the local, regional and global scales (Niehoff *et al.*, 2002). Predictions of ungauged or poorly gauged catchments under these conditions are highly uncertain. Still, it is foreseeable that estimates of streamflow in ungauged catchments continue to be needed. In

fact, many of the most acute problems concerning water quantity and quality are found in the developing countries, where long data records are not at hand.

Although the problems of flood protection and water resources management continue to be of importance for the security of communities and for human, social and economic development within the Masinga catchment, understanding of hydrological processes in this catchment is not adequate. River systems are the major source of water for agricultural and urban water needs in Kenya, but water quality assessments of the river systems still remain difficult within the Masinga catchment, and there is a concern about the sustainable supply of quality water. There is a need to have a monitoring system in order to assess the effects of different land management practices on water quantity and quality within the major catchments in the country. But, long-term continuous monitoring is not being conducted due to high costs of installing and maintaining the gauging equipment. Hence, there is a need for an alternate tool such as a catchment-scale hydrological model that is capable of predicting the streamflow changes as a result of land management within the Masinga catchment.

There are several hydrological models in existence today. These differ mostly in the hydrological variables of concern and in the space–time region of model applicability. Some of these models tend to concentrate on the catchment as the basic hydrological unit (control volume, cf. Bras, 1990). Most hydrological models are traditionally based on deductive cause-effect relationships developed for the temperate regions. However, the sustainable management of vulnerable regions in other climates, such as the tropics, demands a holistic approach relying on first principles and integration of processes and landscape patterns (Gumbrecht *et al.*, 1997). Considering the requirements of various models and their limitations in the ability to predict hydrological changes with land-use transformations, one can conclude that traditional models are not readily applicable in most tropical developing countries.

A number of physically-based distributed models of the hydrological cycle have been successfully integrated with geographical information systems (GIS). The availability of remote sensing data and application of GIS provide very useful input data requirement for physically-based hydrological models (He, 2003). The use of remote sensing and GIS facilitates analyses of large scale, complex and spatially distributed hydrological data. The advanced modelling techniques have become feasible because the time-consuming data manipulations can now be generated efficiently with GIS spatial operations.

Various methods for creating GIS-based models of hydrological processes are emerging but they have not yet been standardized to the point that they can be applied widely. The integration of hydrological processes, particularly integration of surface and groundwater flow, has not been adequately solved yet (Maidment, 1996). However, the advent of object-oriented GIS programming languages has broken the barrier to capturing time variation of spatial processes—a limitation in earlier GIS applications to hydrology (Ye, 1996). As one example of this approach, Ye (1996) presented the development of a subsurface and surface hydrological model entirely within the ArcView GIS.

## Objective

The objective of this study is to apply the Stream Flow Model as an alternative tool for predicting the daily streamflow for the river reaches within the Masinga catchment based on the available remotely sensed data and local data sets.

## STUDY AREA

The Masinga catchment area is some 6255 km<sup>2</sup>, lying to the east of the Aberdare Mountains and south of Mount Kenya. It lies between 0°7'–1°15'S and 36°33'–37°46'E. The geology of the Masinga area can be broadly divided into volcanic rocks in the north and west, and pre-Cambrian basement complex in the southeast. The landform in the catchment ranges from steep mountainous terrain with strong relief in the west, to undulating plains with subdued relief in the southeast. The elevations range from 2500 to 4000 m and from 900 to 1200 m (a.m.s.l.) for the mountainous terrain and the undulating plains, respectively. The soils are generally Lithosols and Histosols (FAO classification) at the highest altitudes in the Aberdare, with Humic Andosols at lower elevations.

The catchment falls within five agro-climatic zones of Kenya, ranging from semi-arid in the east to humid near the western side. The annual rainfall is bimodal with low and high rainfall occurring between September and November, and between March and May, respectively. The mean annual rainfall varies from about 600 mm on the easterly boundary to over 2000 mm on the Aberdare Mountains. The maximum and minimum temperature varies between 25.5–31.0°C and 21.0–24.0°C, respectively. The catchment has an estimated population of 2 million people (1999 population census). The agricultural and grazing activities take about 85% of the total catchment area (Saenyi, 2002).

## MATERIALS AND METHODS

### Model selection

Recognizing the need for simplified catchment models within a GIS environment, this study focused on creating an infiltration-excess runoff model entirely within ArcView GIS and used it as a tool for predicting daily streamflow within the Masinga catchment. A collection of Avenue scripts forms the basis for the raster-based model and uses the capabilities of the Spatial Analyst and the Hydrologic Modeling Extensions. ArcView GIS provides a common interface for model pre-processing of catchment and hydrometeorological data, model setup and execution as well as post-processing of model output, including runoff maps, soil water conditions and the stream hydrograph. The model chosen for this study is the geospatial Stream Flow Model (SFM). The model couples a runoff generation subcomponent based on the Soil Conservation Service (SCS) approach and a DEM-based Travel Time routing method. Although the SFM generates the runoff based on the curve number, the approach employed in this study was to use a grid-based curve number layer. This was used to provide for process dynamics of the study area.

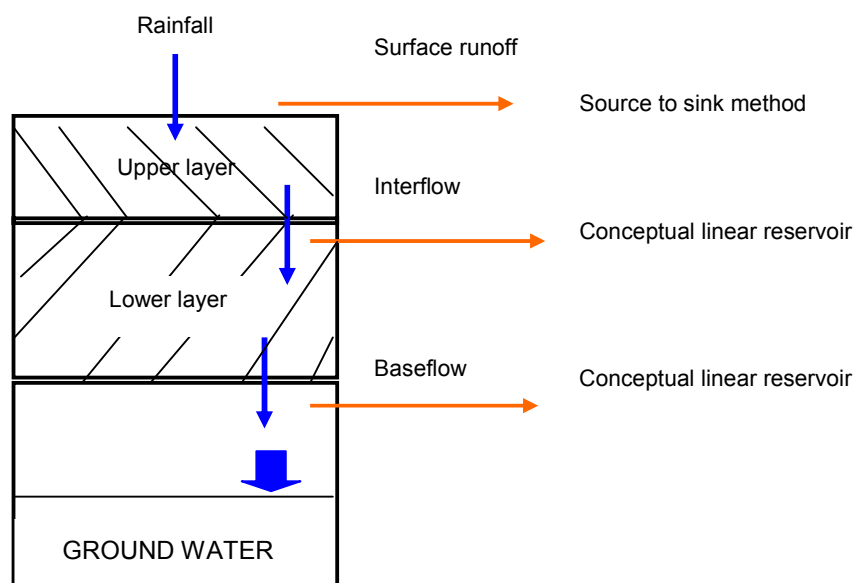
**Stream Flow Model (SFM)** The SFM is a geospatial area-wide flood hazard monitoring system, developed using the “C” programming language by the US Geological Survey (USGS). The user interface was developed using the Environmental Systems Research Institute (ESRI) ArcView GIS software. The hydrological component of the SFM system is a physically-based, catchment-scale model that simulates the dynamics of runoff processes by utilizing remotely-sensed, widely available global

and local data sets. In terms of input data requirements, the model is parsimonious. The basic unit of the SFM is the sub-catchment, which is the subject of a daily water balance calculation.

The SFM has an upland headwater basin routing module, and a major river channel routing module. The model determines how much water enters the stream network from each sub-catchment. For each time step (one day for the SFM), the model computes streamflow, soil water content, and water depth in the main channel for each identified sub-catchment and routes streamflow between those sub-catchments. The default precipitation value is zero and any day when the precipitation is 0.1 mm or less the daily precipitation is set to zero. The default evaporation value is 0.5 mm and is used when the input value is 0.5 mm or less. The model first determines the excess precipitation, that is, the amount of precipitation falling on the catchment that cannot infiltrate into the soil or be used by the evapotranspiration processes. Within the sub-catchments, surface runoff is simulated using a source-to-sink method, while subsurface contributions to streamflow are modelled with two conceptual linear reservoirs, as shown in Fig. 1.

In the major river channels, water is routed using a nonlinear Muskingum-Cunge scheme (e.g. Cunge, 1969; Dooge *et al.*, 1982; Wilson, 1990). During each time step, the model simulates the discharge based upon information from the previous step. This information does not exist during the first time step. The model estimates the initial streamflow as being the streamflow needed to achieve a bankfull condition. The model executes a loop to compute the Muskingum weighting coefficient. Once the Muskingum coefficients have been calculated for this time step, the runoff is routed downstream. The first step is to set the initial value to the last known streamflow values. At time-step zero, this is the inflow from the upstream catchment.

Most model parameters have physical meaning determined by the spatial distribution of sub-catchment characteristics. The model can use local geospatial meteorological and GIS data sets to parameterize model parameters and variables. The



**Fig. 1** Routing within each sub-catchment.

meteorological data used by the model can be remotely-sensed or ground-based data. The parameterization of catchment hydrological properties is accomplished through the use of three fundamental GIS data sets describing the Earth's surface, that is, topography, land-cover and soils. The topographic, hydrological network data and land-use data can be obtained from many sources, such as the USGS. The United Nation's Food and Agriculture Organization (FAO) digital soil map provides the soil information. The information contained in the geospatial data sets is organized as input-data files for the model through the use of a graphical user interface (GUI) to ArcView.

### Application of the SFM

The SFM simulates daily streamflow using two primary input-data files. One of the input-data files contains parameter values describing the physical characteristics (here referred to as *basin.txt* file) of the sub-catchment being modelled. The other input data files (*rain.txt* and *evap.txt*) contain values for forcing variables describing the daily total precipitation and potential evapotranspiration occurring over the sub-catchments. The data needed to create the input-data files were acquired in electronic form as GIS files. One of the primary functions of the SFM GUI is the creation of the model input-data files using these GIS files.

### Model factor generation for the Masinga catchment

**Digital elevation model (DEM)** A DEM of 1:250 000 scale and a grid resolution of 90 m × 90 m covering the study area was purchased from the USGS. The DEM was projected using the Universal Transverse Mercator 37 North (UTM-37N) reference system. The DEM was corrected for the artificial "sinks". To calculate a drainage network, a grid must exist that is coded for the direction in which each cell in a surface drains. The flow direction for this study was based on the eight-direction pour point (8D) (e.g. Moore *et al.*, 1994; Burrough & McDonell, 1998). The 8D algorithm identifies the grid cells, out of the eight surrounding cells towards which water will flow if driven by gravity. The algorithm uses a moving 3 × 3 cell neighbourhood to assign a flow direction to the cell in the centre by considering the direction of the largest drop in elevation.

In order to generate a drainage network for the study area, the ultimate flow path of every cell on the landscape grid was determined. This was done by generating the flow accumulation data layer, which defines the amount of upstream area draining into each cell (Mark *et al.*, 1984; O'Callaghan & Mark, 1984; Martz & Garbrecht, 1993). A threshold value of 500, which is a value that defines the number of grid cells that must flow through an area to be called a stream, was selected for this study. The model spatial framework, catchment boundaries and stream networks, were generated from the digital elevation model data (DEM) using the standard delineation procedure for ArcView (ESRI, 1996).

The delineated catchment was discretized into morphological units (i.e. areas of defined aspect, length, steepness). These were aggregated into seven major sub-catchments (Fig. 2) based on the pour points (outlets) of the delineated stream network.

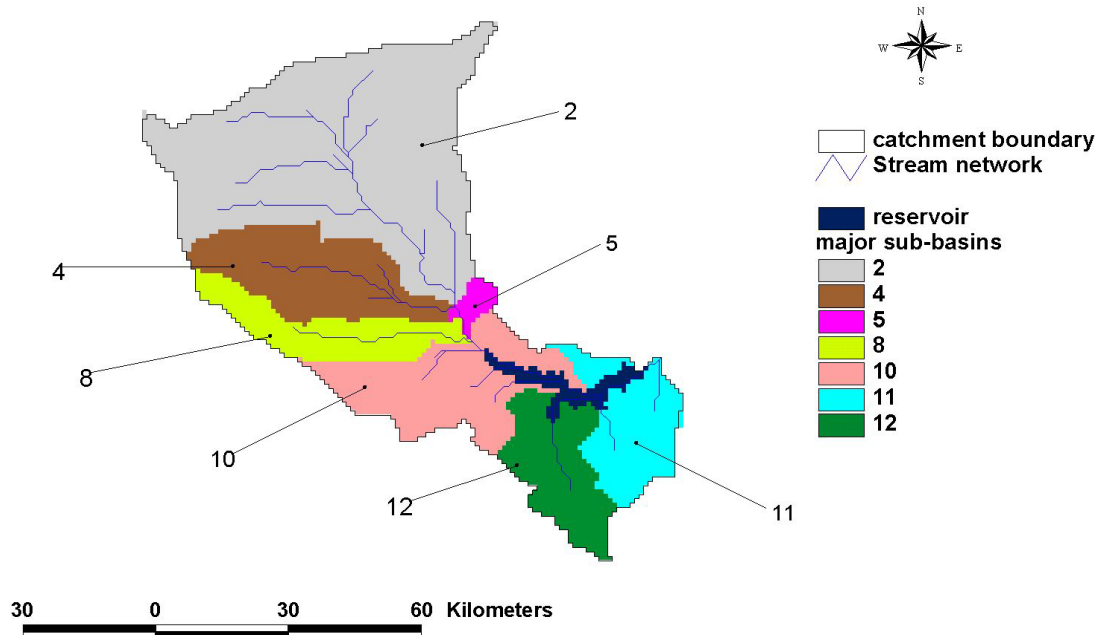


Fig. 2 Discretized sub-catchments as grid layers.

**Land-use/land-cover data** Many studies have shown that the land uses within a catchment can account for much of the variability in streamwater quantity and quality (e.g. Hunsaker *et al.*, 1992; Roth *et al.*, 1996). The percentage and location of natural land cover influence the amount of energy that is available to move water and materials (Hunsaker & Levine, 1995). For instance, forested catchments dissipate energy associated with rainfall, whereas catchments with bare ground cover are less able to do so. A drastic change in vegetation cover through clearing and increased agricultural practices without proper conservation measures can produce more runoff (Franklin, 1992).

In this study, a digital land-cover data set for the Masinga catchment was clipped from the land-use/land-cover map of Kenya derived from a twelve-month series of 1-km vegetation index imagery (Loveland & Belward, 1997). This was used because no recent data for the Masinga catchment were available at a finer resolution. The land cover was further classified to reflect the main land-use/land-cover categories in the Masinga catchment. Using the land-use/land-cover in conjunction with soil information, rainfall incident on each sub-catchment was partitioned to separate surface runoff from water infiltrating into the soil. The land-use/land-cover and soil data were also used in SFM to calculate the response function of each sub-catchment, describing how excess precipitation was routed to the outlet of the sub-catchment.

**Soil data** The SFM required data describing the average water holding capacity of the soil (cm), average hydrologically active soil depth in (cm), textural description of the soil class, average saturation soil hydraulic conductivity in ( $\text{cm h}^{-1}$ ), average Soil Conservation Service (SCS) curve number for the soils, maximum and minimum percentage of the impervious catchment area for each sub-catchment. These parameters were extracted from the Digital Soil Map of Kenya (FAO-UNESCO, 1998) by clipping the study area. The main average physical properties for the discretized sub-catchments and some of these parameters are summarized in Table 1.

**Table 1** Main average attributes of the discretized sub-catchments.

Basin ID	Soil WHC (mm)	Soil depth (cm)	Area (km <sup>2</sup> )	Hlength (m)	Hslope (m)	UpArea (km <sup>2</sup> )	Elevation (m)	SCS curve number	Max cover	Manning coeff.
2	117.0	94.0	2758	21776.3	1.6912	2757	2143.9	76.4	0	0.065
4	127.0	101.0	821	23002.3	1.9787	820	1897.4	73.3	0	0.045
5	64.0	103.0	76	5312.6	0.6324	3654	1198	79.8	0	0.025
8	108.0	98.0	506	34384	1.865	505	1802.5	73.1	0	0.035
10	78.0	122.0	918	16939.1	0.901	5078	1309.9	76.9	0.00106	0.035
11	113.0	195.0	597	13419.6	0.874	6261	1121	73.9	0.00147	0.075
12	89.0	150.0	586	18397.8	0.9661	585	1213.9	75.4	0.00106	0.055

WHC: water holding capacity; Hlength: hillslope length; Hslope: hillslope (m/100m); UpArea: upslope contributing area; Max cover: maximum % cover of land that is impervious

**Table 2** SCS curve numbers in relation to land cover classes and hydrological soil groups.

Land cover description	Hydrological soil group:			
	A	B	C	D
Urban and built-up land	73	82	88	90
Dryland cropland and pasture	71	80	86	86
Irrigated cropland and pasture	64	74	81	84
Cropland/grassland mosaic	63	73	82	87
Cropland/woodland mosaic	51	68	78	82
Grassland	60	76	81	89
Shrubland	48	62	73	78
Savanna	44	65	77	82
Deciduous broadleaf forest	55	66	74	79
Evergreen broadleaf forest	55	66	74	79
Water bodies	100	100	100	100
Herbaceous wetland	100	100	100	100
Wooded wetland	100	100	100	100
Barren or sparsely vegetated	75	80	85	90

In this study, the spatial SCS curve numbers were determined by first merging the hydrological soil groups and land-use/land-cover shape files to form a common field. The ArcView “field calculator” was then used to estimate the curve numbers based on the relationship between the SCS curve numbers, the four hydrological soils groups, and land-use/land-cover given in Table 2.

**Rainfall and potential evapotranspiration estimates** Daily variations in weather drive the calculation of the streamflow estimates. Fluxes of water between the atmosphere and the Earth’s surface are described using geospatial estimates of precipitation and evaporation. A number of methods can be used to estimate rainfall and evapotranspiration. For instance, the rainfall can either be estimated using the satellite rainfall estimates (RFE) or station data. The RFE describe the spatial distribution of precipitation, which the SFM can use to determine the gross input of water to each sub-catchment for each day.

In this study, data from geo-referenced weather stations within the Masinga catchment (Fig. 3) were used. The rainfall and evaporation data for these stations were obtained from the Meteorological Department in Kenya. The daily rainfall and



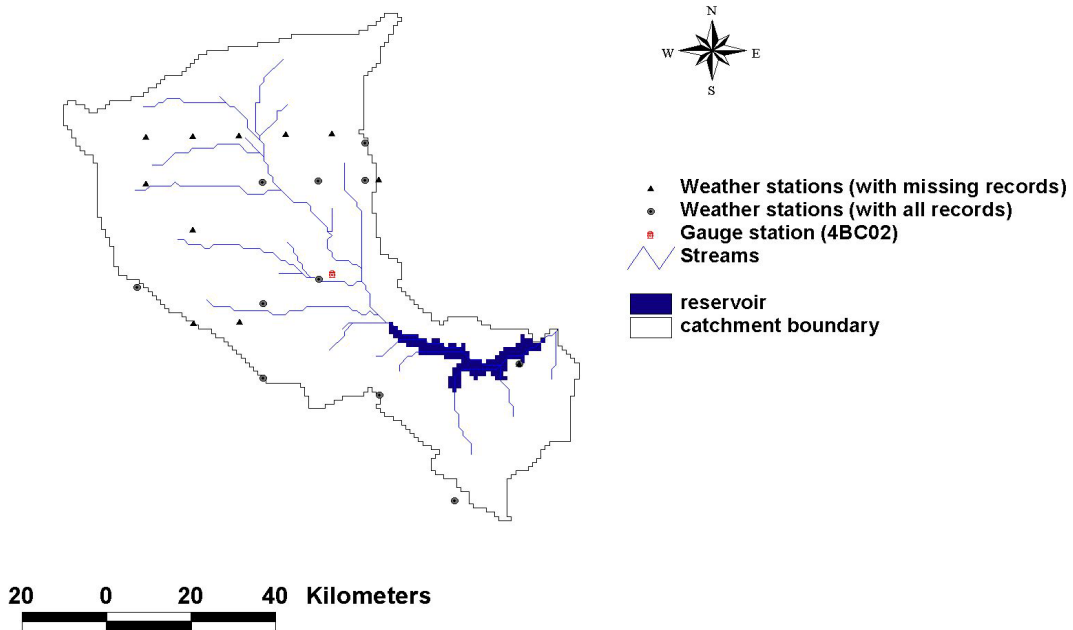


Fig. 3 Delineated catchment showing the stream network, rainfall stations and gauging station.

evaporation were interpolated using the inverse distance weighting (IDW) method and the spatial grids were generated into *rain.txt* and *evap.txt* files, a format used by the SFM. These text files together with the *basin.txt* file derived earlier from the DEM were used as the input data for the model.

### Estimation of runoff grid layer

The surface runoff (excess rainfall) was estimated using the SCS curve number method. This was based on the relationship:

$$Q_i = \frac{(P_i - 0.2S_i)^2}{P_i + 0.8S_i} \text{ for } P > 0.2S \tag{1}$$

where subscript *i* refers to the *i*th cell,  $Q_i$  (mm) is the daily runoff,  $P_i$  (mm) is the daily rainfall and  $S_i$  (mm) is the retention parameter estimated using the relationship:

$$S_i = 254 \left( \frac{100}{CN_i} - 1 \right) \tag{2}$$

where  $CN_i$  is the grid curve number.

The overland flow velocity was estimated using the kinematic wave equation. A modified velocity equation was used based on land-use type and land slope. Values of velocity coefficient  $\alpha_i$  for each land-use/land-cover type were estimated using values given in Table 3. The velocity was estimated using the relationship:

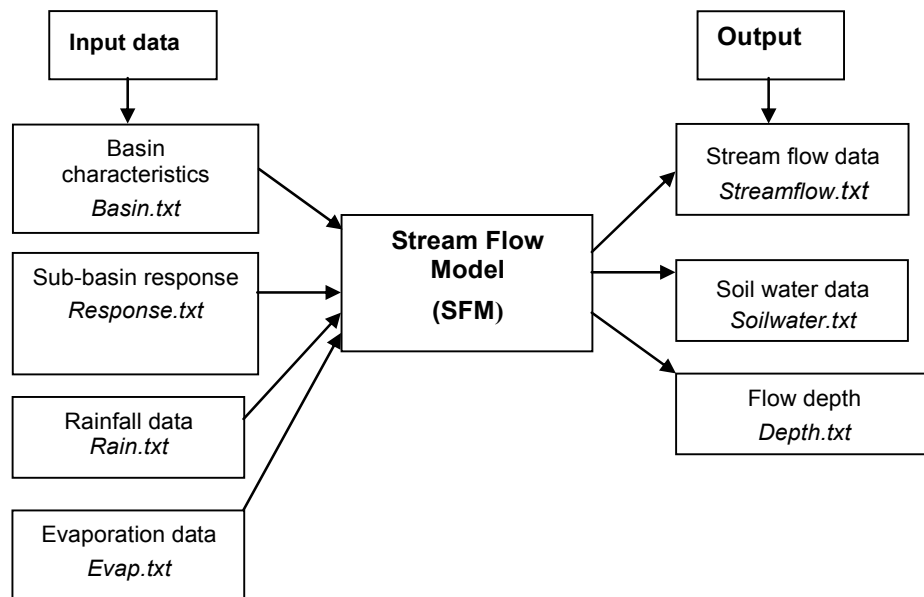
$$v_i = (\alpha_i s_i^{1/2}) q_i \tag{3}$$

where  $v_i$  is the runoff velocity ( $\text{m s}^{-1}$ ),  $s_i$  ( $\text{m m}^{-1}$ ) is the slope of cell *i* and  $q_i$  ( $\text{m s}^{-1}$ ) is the specific runoff rate (i.e. runoff rate per unit cell area).

**Table 3** Relationship between land cover description and velocity coefficient.

Land cover description	Velocity coefficient
Unknown land use	0.0000
Urban and built-up land	6.3398
Dryland cropland and pasture	0.4572
Irrigated cropland and pasture	2.7737
Cropland/grassland mosaic	0.3962
Cropland/woodland mosaic	0.3962
Grassland	0.6401
Shrubland	0.4572
Savanna	0.4267
Deciduous broadleaf forest	0.4267
Evergreen broadleaf forest	0.2134
Water bodies	14.1122
Herbaceous wetland	4.7854
Wooded wetland	3.1394
Barren or sparsely vegetated	0.6706

(after Maidment *et al.*, 1996 and McCuen, 1998).

**Fig. 4** Conceptual framework for the Stream Flow Model.

### Running the Stream Flow Model (SFM)

The conceptual framework of the SFM model is given in Fig. 4. After the required grids were created, they were generated into text files (.txt), a format used in the SFM. The most important text files were the *basin.txt*, *response.txt*, *rain.txt* and *evap.txt*. The SFM graphical user interface (GUI) has a menu from which different functions can be selected. Using the SFM menu, a number of outputs were simulated, some of which included the daily streamflow and the soil water conditions for the discretized sub-catchment as given in Fig. 5.

### Stream Flow Model verification

The SFM performance was verified using the observed and simulated daily streamflow data for 1992 at Tana-Sagana gauging station (number 4BC02) (see Fig. 3). The year 1992 was chosen because of the availability of daily rainfall and evaporation records for the whole year for all weather stations considered in the study area. The Tana-Sagana gauging station was chosen because it is the only gauging station with complete daily streamflow records for 1992.

## RESULTS AND DISCUSSION

The model was able to simulate streamflow and soil moisture conditions as an indicator of the hydrological conditions existing within each sub-catchment. The results in Fig. 5 show the variation of daily streamflow and soil water conditions within each sub-catchment. These results were modelled from the land-use and climatic conditions for 1997. From Fig. 5, it can be seen that daily streamflow and soil water conditions vary significantly across the sub-catchments. This can be attributed to the varied land-use practices, soil characteristics and the agro-climatic zone for each sub-catchment.

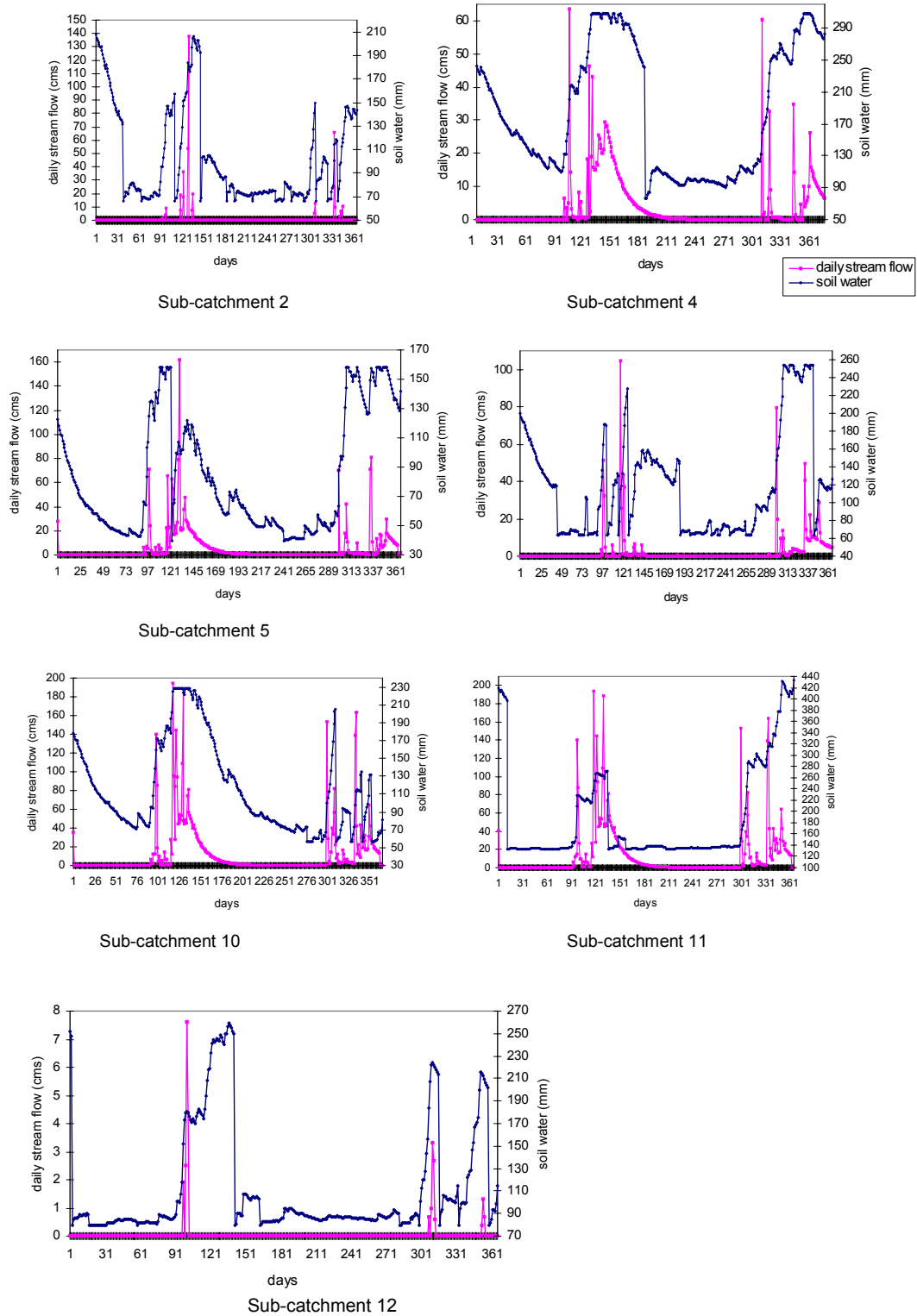
The model performance was verified by comparing the observed and simulated streamflow data for Tana-Sagana gauging station (4BC02), as shown in Fig. 6. From the results, it can be seen that the simulated and observed daily streamflow data compare fairly well. The predicted model results give a similar trend of the observed daily streamflow. The flows are highest during April and May. This coincides with the highest rainfall period. Moderately high flows are also recorded during November and December, the period of low rainfall. The model slightly overpredicted daily streamflow during the high rainfall season and underestimated the flows during the low rainfall season. However, from the results (Fig. 6), it can be seen that the model can be used to estimate the daily streamflow.

A statistical criterion for evaluating hydrological goodness of fit between the measured (observed) and predicted (simulated) values was applied. The results were compared using the coefficient of model efficiency (COE) according to Nash & Sutcliffe (1970), given as:

$$\text{COE} = 1 - \left( \frac{\sum_i^n (q_i - q_s)^2}{\sum_i^n (q_i - q_m)^2} \right) \quad (4)$$

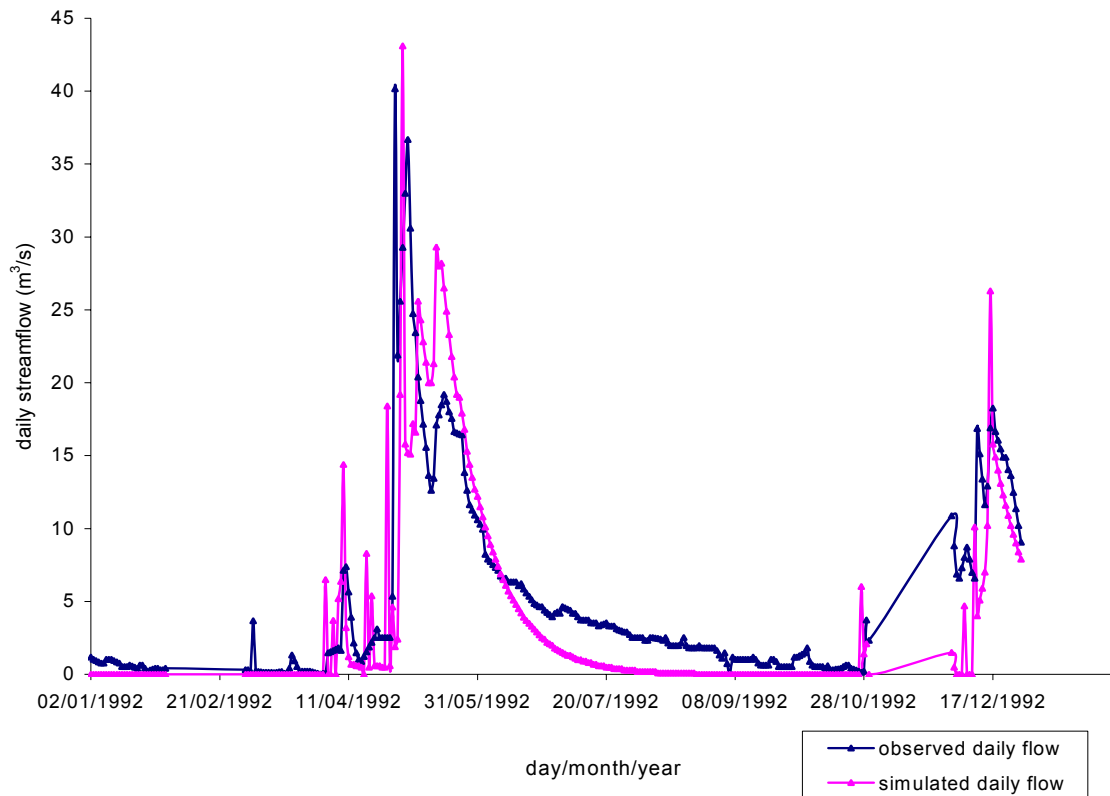
where COE is the coefficient of efficiency,  $q_i$  is the observed (measured) daily streamflow ( $\text{m}^3 \text{s}^{-1}$ ),  $q_s$  is the simulated (predicted) daily streamflow ( $\text{m}^3 \text{s}^{-1}$ ),  $n$  is the number of observations and  $q_m$  is the mean observed daily flow ( $\text{m}^3 \text{s}^{-1}$ ). From the results, a COE of 0.74 was obtained. The best coefficient value should be about 1.0 and the value of 0.74 shows that the simulated and observed flows have a moderate correlation and hence the model can be relied upon.

It should be noted that most of the grid layers used as the input data to the SFM to simulate the daily flows were of coarse resolution and this, of course, compromised the model performance. In addition to coarse input data, these results are based on the



**Fig. 5** Daily streamflow and soil water for the major sub-catchments in the Masinga catchment.

initial model parameters before the model was calibrated. It is envisaged that by varying some of the model parameters, such as the runoff curve number, soil water



**Fig. 6** Observed and simulated daily streamflow at Tana-Sagana gauging station (4BC02) in 1992.

holding capacity, soil water depth and the saturated hydraulic conductivity through calibration, the correlation could be significantly improved, thus improving the prediction capability of the model.

## CONCLUSIONS

This study presents a first attempt in the application of GIS technology to predict daily streamflow in the Masinga catchment. It demonstrates the integration of a physically-based hydrological model, the SFM, within the ArcView GIS environment to estimate daily streamflow for ungauged river reaches in a large rural catchment. The GIS was used to prepare the required spatial data, extract input parameters for the model, execute the model computations, query and display results. The GIS therefore provided a fast and efficient means of generating the input data required for the model. With the availability of remotely sensed data, global and local data sets, the tool can be used by government agencies and catchment stakeholders to continuously monitor the daily streamflow within the catchment.

The results from the preliminary application presented in this paper show a promising alternative method for predicting daily streamflow in ungauged catchments. However, there is a need for more field work to collect data for the model calibration and validation. It is envisaged that the capability of the model to predict daily streamflow will be improved once it is calibrated and validated.

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